A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

November 2009

2009 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M, 02-07 10M, 02-08 10M)

Page(s)	**Revision**	
64	$Defl_{Max}$ = span · 12 in/ft / ~~240~~ 360	change 240 to 360 in the denominator
	$Defl_{Max}$ = ~~0.9~~ 0.6 in	
65	$Defl_{Max}$ = span · 12 in/ft / ~~240~~ 360	change 240 to 360 in the denominator
	$Defl_{Max}$ = ~~0.9~~ 0.6 in	
66	I_{reqd} = ~~1037~~ 1556 in^4	
	Minimum required I is ~~1037~~ 1556 in^4 for deflection	
67	I_{reqd} = ~~1037~~ 1556 in^4	
	Minimum required I is ~~1037~~ 1556 in^4 for deflection	

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org
America's Forest & Paper People® - *Improving Tomorrow's Environment Today®*

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

September 2009

2009 ERRATA/ADDENDUM
to the

2005 Edition of

the *National Design Specification*® *(NDS®) for Wood Construction Supplement:*
Design Values for Wood Construction
(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

Page	**Revision**
60	Revise footnote 4 of Table 5A, footnote 3 of Expanded Table 5A, and footnote 3 of Table 5B as follows:

The design value for shear, $\mathbf{F_{vx}}$ and $\mathbf{F_{vy}}$, shall be decreased by multiplying by a factor of 0.72 for non-prismatic members, notched members, and for all members subject to impact or cyclic loading. The reduced design value shall be used for design of members at connections (NDS 3.4.3.3 and 10.1.2) that transfer shear by mechanical fasteners. The reduced design value shall also be used for determination of design values for radial tension (NDS 5.2.2).

AF&PA®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

July 2009

2009 ERRATA/ADDENDUM
to the

2005 Edition of

the Commentary to the *National Design Specification® (NDS®) for Wood Construction*
(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

Page	**Revision**
249	Revise equation C15.1-1 as follows:
	$P_{14} = -180.7 + 140.5 \log(P_m)$

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

November 2008

2008 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed versions dated 02-08-10M, 08-06 5M, and 02-07 10M)

Page(s)	Revision	
264	$M_{Total} = M_{load} + Weight \cdot Length^2 / 8 \cdot \sout{12}$	delete "12" in the denominator
	f_b = ~~1695~~ <u>1778</u> psi	
265	$M_{Total} = M_{load} + 1.2(Weight \cdot Length^2) / 8 \cdot \sout{12}$	delete "12" in the denominator
	f_b = ~~2575~~ <u>2674</u> psi	
274	$M_{Total} = M_{load} + Weight \cdot Length^2 / 8 \cdot \sout{12}$	delete "12" in the denominator
	f_b = ~~1352~~ <u>1401</u> psi	
275	$M_{Total} = M_{load} + \underline{1.2}(Weight \cdot Length^2) / 8 \cdot \sout{12}$	delete "12" in the denominator and incorporate 1.2 load factor
	f_b = ~~2145~~ <u>2204</u> psi	

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

September 2008

2008 ERRATA/ADDENDUM
to the

2005 Edition of

the National Design Specification® (NDS®) for Wood Construction
(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

The 1991-2005 Editions of the *National Design Specification®(NDS®) for Wood Construction* have contained provisions for designing columns loaded simultaneously with axial, edgewise bending, and/or flatwise bending loads using a stress interaction equation. When a flatwise bending load is checked with the third term of the stress interaction equation, the axial and edgewise bending interaction in the denominator can become a negative value. The occurrence of the negative value indicates an overstress. Use of this negative term in the stress interaction equation overlooks the overstress in flatwise bending and incorrectly reduces the overall interaction.

While a check for overstress due to bending is a limiting condition of member design for bending per 3.3.1 of the *NDS*, an explicit limit is provided to clarify limitations on flatwise bending in *NDS* stress interaction equations as follows:

Page **Revision**
20-21 Add limitation to provisions in *NDS* 3.9.2:

$$\frac{f_c}{F_{cE2}} + \left(\frac{f_{b1}}{F_{bE}}\right)^2 < 1.0$$

Page **Revision**
190 Append to *NDS Commentary* C3.9.2:

The limits on f_c and f_{b1} (e.g. $f_c<F_{cE1}$, $f_c<F_{cE2}$, and $f_{b1}<F_{bE}$) do not address the case where the sum of the f_c/F_{cE2} stress ratio and the square of the f_{b1}/F_{bE} stress ratio in the denominator of the third term of Equation 3.9-3 exceeds 1.0. In this case, the third term becomes negative indicating overstress in flatwise bending due to combined loading effects. Inclusion of the third term as a negative value overlooks the overstress in flatwise bending and incorrectly reduces the interaction calculation. A limit on $(f_c/F_{cE2}) + (f_{b1}/F_{bE})^2$ is stated explicitly to clarify limitations on flatwise bending in the stress interaction equation and avoid accidental inclusion of a negative value in the stress interaction equation.

Page **Revision**
137 Add limitation to provisions in *NDS* 15.4.1:

$$\frac{f_c}{F_{cE2}} + \left(\frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}}\right)^2 < 1.0$$

Page	**Revision**
253	Append to *NDS Commentary* C15.4.1:

The limits on f_c and f_{b1} (e.g. $f_c<F_{cE1}$, $f_c<F_{cE2}$, and $f_{b1}<F_{bE}$) do not address the case where the sum of the f_c/F_{cE2} stress ratio and the square of the f_{b1}/F_{bE} stress ratio or the $[f_{b1}+ f_c(6e_1/d_1)]/F_{bE}$ ratio in the denominator of the third term of Equations 15.4-1 and 15.4-2 exceeds 1.0. In this case, the third term becomes negative indicating overstress in flatwise bending due to combined loading effects. Inclusion of the third term as a negative value overlooks the overstress in flatwise bending and incorrectly reduces the interaction calculation. A limit on $(f_c/F_{cE2}) + ([f_{b1}+ 6f_ce_1/d_1]/F_{bE})^2$ is stated explicitly to clarify limitations on flatwise bending in the stress interaction equation and avoid accidental inclusion of a negative value in the stress interaction equations.

AF&PA®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

September 2008

2008 ERRATA
to the

2005 Edition of

the *SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC, ANSI/AF&PA SDPWS-2005*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M, 02-07 10M, and 02-08 10M)

Page(s)	Revision	
47	In Example C4.2.2-1 revise "Nail load/slip at 1.4 $v_{s(ASD)}$:"	Subscript (ASD)
	In Example C4.2.2-1 revise "= $(129.5/456)^{3.144}$ = 0.0191"	Superscript 3.144
50	In Equations C4.3.2-1 and C4.3.2-2 revise "(tie-down ~~nail~~ slip)"	Delete "nail"
	In the third paragraph of C4.3.2 (second column) revise "…or 3-term equation (SDPW~~D~~S)…"	Change "D" to "S"
53	In Example C4.3.2-1, text beginning with "C4.3.2.1" to the end should be moved to page 51 at the end of section C4.3.2.	

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

A F & P A

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

September 2008

2008 ERRATA/ADDENDUM
to the

2005 Edition of

the *ASD/LRFD MANUAL FOR ENGINEERED WOOD CONSTRUCTION*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M, 02-07 10M, and 02-08 10M)

Page(s)	**Revision**
155	Revise Table M16.2-10 column headers for decking width as follows:

Rating	1-HOUR				1.5-HOUR			2-HOUR	
Decking Width	~~5~~1.5	~~7~~2.5	~~9~~3.5	~~11~~5.5	~~7~~2.5	~~9~~3.5	~~11~~5.5	~~9~~3.5	~~11~~5.5

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org
America's Forest & Paper People® *- Improving Tomorrow's Environment Today* ®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

April 2008

2008 ERRATA
to the

2005 Edition of

the *ASD/LRFD MANUAL FOR ENGINEERED WOOD CONSTRUCTION*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M, 02-07 10M, and 02-08 10M)

Page(s)	Revision
64	Figure M9.2-4 should be renumbered M9.2-5 Figure M9.2-5 should be renumbered M9.2-4

A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION

American Wood Council

Engineered and Traditional Wood Products

February 2008

2008 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M and 02-07 10M)

Page(s)	Revision	
72	C_{M_Fb} := ~~0.85~~ 1.0	Change wet service factor. According to the *NDS Supplement*, when $(F_b)(C_F) \leq 1150$ psi $C_M = 1.0$
	F'_{bx} = ~~1124~~ 1322 psi	This change does not affect the outcome - bending still checks.
73	C_{M_Fb} := ~~0.85~~ 1.0	Change wet service factor. According to the *NDS Supplement*, when $(F_b)(C_F) \leq 1150$ psi $C_M = 1.0$
	F'_{bx} = ~~2428~~ 2856 psi	This change does not affect the outcome - bending still checks.

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

October 2007

2007 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M and 02-07 10M)

Page(s)	Revision
91	λ = ~~1.0~~ 0.8 Time effect factor
93	$F'_{bS\_24\_0}$ = ~~648~~ 518 lbf·in $F'_{bS\_24\_16}$ = ~~832~~ 665 lbf·in $F'_{bS\_32\_16}$ = ~~961~~ 769 lbf·in $F'_{bS\_40\_20}$ = ~~1620~~ 1296 lbf·in $F'_{bS\_48\_24}$ = ~~2160~~ 1728 lbf·in **The maximum moment is 826 in-lb/ft, so the ~~24/16~~ 40/20 span rating works for flexure.**
95	Check the ~~24/16~~ 40/20 span rating for shear. F_{s_IbQ} := ~~150~~ 205 lbf F'_{s_IbQ} = ~~324~~ 354 lbf **The ~~24/16~~ 40/20 span rating is OK in shear.** EI_{40_20} := ~~78000~~ 225000 lbf·in^2 — EI = 225000 ~~78000~~ lbf in^2 per foot of width for ~~24/16~~ 40/20 span rating. Table M9.2-1. EI'_{40_20} := ~~78000~~ 225000 lbf·in^2 — ~~24/16~~ 40/20 bending stiffness
96	Δ < L/240 = ~~0.95~~ 0.095 in, thus deflection is OK
97	$\Delta = 0.00677\ w_{load}\ span_{defl}^4 / EI'_{40\_20}$ Δ = ~~0.213~~ 0.074 in

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org
America's Forest & Paper People®* - *Improving Tomorrow's Environment Today®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

May 2007

**2007 ERRATA
to the
2005 Edition of**

**the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package***
(printed versions dated 08-06 5M and 02-07 10M)

Page(s)	Revision
376	$Weight_{curved}$ = π · R_{center}~~/2~~ · Depth · Width · 40 · lbf/ft^3 (delete "2" in the denominator)
	$Weight_{curved}$ = ~~353~~ 706 lbf
	M_{max} = ~~66244~~ 97288 in·lbf
377	$Weight_{curved}$ = π · R_{center}~~/2~~ · Depth · Width · 40 · lbf/ft^3 (delete "2" in the denominator)
	$Weight_{curved}$ = ~~353~~ 706 lbf
	M_{max} = ~~92742~~ 136204 in·lbf
380	f_b = ~~343~~ 504 psi
	V_{load} = ~~376~~ 553 lbf
	f_v = ~~5~~ 8 psi
	f_r = ~~10~~ 15 psi
381	f_b = ~~481~~ 706 psi
	V_{load} = ~~527~~ 774 lbf
	f_v = ~~7~~ 11 psi
	f_r = ~~14~~ 21 psi

384 $Weight_{curved} = \pi \cdot R_{center}$~~/2~~ $Depth \cdot Width \cdot 40 \cdot lbf/ft^3$ (delete "2" in the denominator)

$Weight_{curved}$ = ~~353~~ 706 lbf

M_{max} = ~~66244~~ 97288 in·lbf

P_{max} = ~~553~~ 906 lbf

385 $Weight_{curved} = \pi \cdot R_{center}$~~/2~~ $Depth \cdot Width \cdot 40 \cdot lbf/ft^3$ (delete "2" in the denominator)

$Weight_{curved}$ = ~~353~~ 706 lbf

M_{max} = ~~92742~~ 136204 in·lbf

P_{max} = ~~774~~ 1268 lbf

388 f_c = ~~5~~ 8 psi

389 f_c = ~~7~~ 11 psi

390 f_{b1} = ~~343~~ 504 psi

$$\left(\frac{f_c}{F_c'}\right)^2 + \frac{f_{b1}}{F_b'\left[1-\left(\frac{f_c}{F_{cE1}}\right)\right]} = \sout{0.314}\ \underline{0.469}$$

391 f_{b1} = ~~481~~ 706 psi

$$\left(\frac{f_c}{F_c'}\right)^2 + \frac{f_{b1}}{F_b'\left[1-\left(\frac{f_c}{F_{cE1}}\right)\right]} = \sout{0.305}\ \underline{0.454}$$

392 $\left(\frac{f_c}{F_c'}\right)^2 = \sout{0.00056}\ \underline{0.00151}$ $\frac{f_{b1}}{F_b'\left[1-\left(\frac{f_c}{F_{cE1}}\right)\right]} = \sout{0.313}\ \underline{0.467}$

393 $\left(\frac{f_c}{F_c'}\right)^2 = \sout{0.00049}\ \underline{0.00132}$ $\frac{f_{b1}}{F_b'\left[1-\left(\frac{f_c}{F_{cE1}}\right)\right]} = \sout{0.304}\ \underline{0.453}$

A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

March 2007

2007 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed versions dated 08-06 5M and 02-07 10M)

Page(s)	Revision
276	F'_v = ~~2/3~~ $F_{vx}\ C_D\ C_M\ C_{t\_Fv}$
	F'_v = ~~200~~ 300 psi
	This change does not affect the final results for shear calculations since $f_v < F'_v$.
277	F'_v = ~~2/3~~ $\lambda\ K_{F\_Fv}\ \phi_v\ F_{vx}\ C_M\ C_{t\_Fv}$
	F'_v = ~~345.6~~ 518.4 psi
	This change does not affect the final results for shear calculations since $f_v < F'_v$.

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org
America's Forest & Paper People® *- Improving Tomorrow's Environment Today®*

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

February 2007

2007 ERRATA
to the

2005 Edition of

the *ASD/LRFD MANUAL FOR ENGINEERED WOOD CONSTRUCTION*
included in the *2005 Wood Design Package*
(printed version dated 08-06 5M)

Page(s)	Revision
126	Change the IIC rating for assemblies "Without Gypsum Concrete" with "Carpet & Pad" from ~~62~~ to 66.
129	Change the STC rating for assemblies "With Gypsum Concrete" with "Carpet & Pad" from ~~68~~[c] to 58[c].

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

February 2007

2007 ERRATA/ADDENDUM
to the

2005 Edition of

the National Design Specification® (NDS®) for Wood Construction Supplement: Design Values for Wood Construction
(printed versions dated 04-05 2M, 09-05 2M, and 08-06 5M)

Page **Revision**

4 In Table 2.1, revise the following:

Species or Species Combination	Species That May Be Included in Combination	Grading Rules Agencies	Design Values Provided in Tables
Coast Sitka Spruce	Coast Sitka Spruce	NLGA	4A, 4D, 4E
Yellow Cedar	Yellow Cedar	NLGA	4A

32 In Table 4A, add the following design values for Coast Sitka Spruce

Species and commercial grade	Size classification	Design values in pounds per square inch (psi)							
		Bending F_b	Tension parallel to grain F_t	Shear parallel to grain F_v	Compression perpendicular to grain $F_{c\perp}$	Compression parallel to grain F_c	Modulus of Elasticity E	E_{min}	Grading Rules Agency
Coast Sitka Spruce[4]									
Select Structural	2" & wider	1300	950	125	455	1200	1,700,000	620,000	NLGA
No. 1 / No. 2		925	550	125	455	1100	1,500,000	550,000	
No. 3		525	325	125	455	625	1,400,000	510,000	
Stud	2" & wider	725	450	125	455	675	1,400,000	510,000	
Construction	2"-4" wide	1050	650	125	455	1300	1,400,000	510,000	
Standard		600	350	125	455	1100	1,300,000	470,000	
Utility		275	175	125	455	725	1,200,000	440,000	

34 In Table 4A, revise the following design values for Northern Species

Species and commercial grade	Size classification	Design values in pounds per square inch (psi)							
		Bending F_b	Tension parallel to grain F_t	Shear parallel to grain F_v	Compression perpendicular to grain $F_{c\perp}$	Compression parallel to grain F_c	Modulus of Elasticity E	E_{min}	Grading Rules Agency
Northern Species									
Select Structural	2" & wider	~~1,000~~ 975	~~450~~ 425	110	350	1,100	1,100,000	400,000	NLGA
No. 1 / No. 2		~~600~~ 625	275	110	350	850	1,100,000	400,000	
No. 3		350	150	110	350	500	1,000,000	370,000	
Stud	2" & wider	475	225	110	350	550	1,000,000	370,000	
Construction	2"-4" wide	700	~~300~~ 325	110	350	1,050	1,000,000	370,000	
Standard		400	175	110	350	875	900,000	330,000	
Utility		175	75	110	350	575	900,000	330,000	

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

Page **Revision**

36 In Table 4A, add the following design values for Yellow Cedar

Species and commercial grade	Size classification	Bending F_b	Tension parallel to grain F_t	Shear parallel to grain F_v	Compression perpendicular to grain $F_{c\perp}$	Compression parallel to grain F_c	Modulus of Elasticity E	Modulus of Elasticity E_{min}	Grading Rules Agency
Yellow Cedar[4]									
Select Structural	2” & wider	1200	725	175	540	1200	1,600,000	580,000	NLGA
No. 1 / No. 2		800	475	175	540	1000	1,400,000	510,000	
No. 3		475	275	175	540	575	1,200,000	440,000	
Stud	2” & wider	625	375	175	540	650	1,200,000	440,000	
Construction	2”-4” wide	925	550	175	540	1200	1,300,000	470,000	
Standard		525	300	175	540	1050	1,200,000	440,000	
Utility		250	150	175	540	675	1,100,000	400,000	

Design values in pounds per square inch (psi)

36 In Table 4A, add the following footnote:

4. **SPECIFIC GRAVITY, G.** Specific gravity values are provided below for visually graded dimension lumber. Note that the value for Coast Sitka Spruce is applicable only for visually graded dimension lumber (2” – 4” thick). See NDS Table 11.3.2A for the specific gravity value applicable to Coast Sitka Spruce used as visually graded timber (5”x5” and larger) and visually graded decking.

Species	**Specific Gravity, G**	**Grading Rules Agency**
Coast Sitka Spruce	0.43	NLGA
Yellow Cedar	0.46	NLGA

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

February 2007

2007 ERRATA
to the

2005 Edition of

the *SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC, ANSI/AF&PA SDPWS-2005*
included in the *2005 Wood Design Package*
(printed version dated 08-06 5M)

Page(s)	Revision
6	x = distance from chord splice to nearest support, ~~in.~~ ft
13	x = distance from chord splice to nearest support, ~~in.~~ ft
23	C_o = shear capacity adjustment factor from Table ~~4.3.2.1~~ 4.3.3.4
44	x = distance from chord splice to nearest support, ~~in.~~ ft
46	x = distance from chord splice to nearest support, ~~in.~~ ft

1111 Nineteenth Street, NW, Suite 800 - Washington, DC 20036 - 202 463-4713 - Fax: 202 463-2791 - www.awc.org

A F & P A®

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
Engineered and Traditional Wood Products

January 2007

2007 ERRATA
to the

2005 Edition of

the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS*
included in the *2005 Wood Design Package*
(printed version dated 08-06 5M)

Page(s)	**Revision**
120-121	The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\frac{f_{b2}+f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}\le 1.0$$

The square symbol in the denominator goes between the last 2 brackets in the equation.

132	The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\left(\frac{f_c}{F_c'}\right)^2+\frac{f_{b1}+f_c(6e_1/d_1)[1+0.234(f_c/F_{cE1})]}{F_{b1}'\left[1-(f_c/F_{cE1})\right]}+\frac{f_{b2}+f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}$$

= ~~0.945~~ 0.927

The square symbol in the denominator goes between the last 2 brackets in the equation. This changes the overall result to 0.927 (above) and 0.101 for the minor axis bending term (below).

$$\frac{f_{b2}+f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}$$

= ~~0.119~~ 0.101

Page	**Revision**
133	The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\left(\frac{f_c}{F_c'}\right)^2 + \frac{f_{b1} + f_c(6e_1/d_1)[1+0.234(f_c/F_{cE1})]}{F_{b1}'\left[1-(f_c/F_{cE1})\right]} + \frac{f_{b2} + f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}$$

= ~~0.785~~ <u>0.772</u>

The square symbol in the denominator goes between the last 2 brackets in the equation. This changes the overall result to 0.772 (above) and 0.142 for the minor axis bending term (below).

$$\frac{f_{b2} + f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}$$

= ~~0.156~~ <u>0.142</u>

284-285	The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\frac{f_{b2} + f_c(6e_2/d_2)\left\{1+0.234(f_c/F_{cE2})+0.234\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}}{F_{b2}'\left\{1-(f_c/F_{cE2})-\left[\frac{f_{b1}+f_c(6e_1/d_1)}{F_{bE}}\right]^2\right\}} \le 1.0$$

The square symbol in the denominator goes between the last 2 brackets in the equation.