



March 2002

2002 Errata
to
1996 Edition of

*Load and Resistance Factor Design Manual (LRFD) for Engineered Wood Construction with the
 Standard for Load and Resistance Factor Design for Engineered Wood Construction,
 AF&PA/ASCE 16-95*

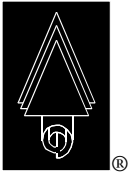
<u>Page</u>	<u>Section</u>	<u>Revision</u>
Page 37	Section 4.3.1	“Sec. 4.3-4” should be “Eq. 4.3-4”
Page 87	Design for shear	“Figure 1” should be “Figure 9.2A”
Page 162	Equation 7.6-9	“ Z_{θ} ” should be “ Z'_{θ} ”
Page 167	First paragraph	“Fig. A2.2-2” should be “Fig. A2.3-1”
Page 169	A2.3.6 Nominal moment resistance	<p>“...M_e, calculated by Eq. 5.4-4.” Should be “...M_e, calculated by Eq. 5.2-7”</p> <p>“M' shall then be calculated by use of Eq. 5.4-1” should be “M' shall then be calculated by use of Eq. 5.2-4”</p> <p>“...and compared with the value obtained from Eq. 5.3-1” should be “...and compared with the value obtained from Eq. 5.2-2.”</p> <p>“The factor for tapering, K_{si}, shall not apply to arches.” should be “The factor for tapering, K_{sr}, shall not apply to arches.”</p>
Page 173	A5.1 Definitions and Notations	“A5.1-1 through A5.1-4” should be “A5.1-1 and A5.1-4”
Page 187	2 nd col. 2 nd par.	“...such as Eq. C1.4-2...” should be “...such as Eq. C1.4-1...”
Page 206	First paragraph in 2 nd column	<p>“Eq. C5.2-7 is compared with Eq. 5.2-8.” should be “Eq. C5.2-3 is compared with Eq. 5.2-7.”</p>
Page 206	Eq. C5-2.4	“ M_b ” in equation should be “ C_b ”

<u>Page</u>	<u>Section</u>	<u>Revision</u>
Page 216	Fig. C7.3-1	“If $s/4 > g$: count as 2 rows of 6 each” should be “If $s/4 > g$: count as 2 rows of 5 each”
Page 219	C7.6.3 2 nd par.	“...Fig. 7.4-1.” should be “...Fig. 7.6-1.”
Page 235	First paragraph	“end block constant, C_{eb} , ...” should be “end block constant, K_s ,...”
Page 237	CA2.3.8 4 th par.	“...by Eq. A2.3.” should be “...by Section A2.3.8”

***LRFD Supplement for Structural Lumber of the
1996 LRFD Manual for Engineered Wood Construction***

<u>Page</u>	<u>Section</u>	<u>Revision</u>
25	Table 3.3 Footnote 1	Add the following: “For members with reference strength values listed in Table 3.3, $C_F = 1.0$ for all properties and all sizes 12” in width and less. For sizes greater than 12”, C_F for F_b is computed as $(12/d)^{1/9} \leq 1.0$, where d is the depth of the member.”
35	Table 4.3 Footnote 1	Add the following: “For dimension lumber wider than 12” (all grades except Dense Structural 86, Dense Structural 72 and Dense Structural 65), tabulated bending, tension and compression parallel to grain design values for 12” wide lumber shall be multiplied by the size factor, $C_F = 0.9$. When the depth, d , of Dense Structural 86, Dense Structural 72 or Dense Structural 65 dimension lumber exceeds 12”, the tabulated bending design value, F_b , shall be multiplied by the following size factor: $C = (12/d)^{1/9} \leq 1.0$.”
35	Table 4.3 Footnote 3	Replace with: “For members with reference strength values listed in Table 3.3, $C_F = 1.0$ for all properties and all sizes 12” in width and less. For sizes greater than 12”, C_F for F_b is computed as $(12/d)^{1/9} \leq 1.0$, where d is the depth of the member.”

Future updates will be available at <http://www.awc.org>

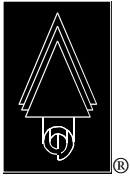


June 1999

**1999 ERRATA
to
1996 Edition of**

**LRFD MANUAL FOR ENGINEERED WOOD CONSTRUCTION with the
STANDARD FOR LOAD AND RESISTANCE FACTOR DESIGN FOR
ENGINEERED WOOD CONSTRUCTION, AF&PA/ASCE 16-95**

<u>Page</u>	<u>Revision</u>
139	In equation 5.1.4, " $\tan \theta$ " in the third term of the denominator should be " $\tan^2 \theta$ ".
139	In paragraph 2 of section 5.1.11, remove "for use in equation 5.1-4".
169	" K_{si} " in A2.2.6 and A2.3.6 should be " C_1 ".
169	In section A2.3.6 reference to equations 5.4-4, 5.4-1, and 5.3-1 should be to equations 5.2-7, 5.2-4, and 5.2-2, respectively.
225	In equation C8.5-3, $(EI)'$ should appear in the denominator where $(EI)'$ is the adjusted flexural stiffness (kip-in. ² /ft). k constants for C8.5-3 should be 922 instead of 0.92, 2220 instead of 2.22, and 1740 instead of 1.74.



January 1999

**1999 ERRATA
to
1996 Edition of**

**LRFD MANUAL FOR ENGINEERED WOOD CONSTRUCTION with the
STANDARD FOR LOAD AND RESISTANCE FACTOR DESIGN FOR
ENGINEERED WOOD CONSTRUCTION, AF&PA/ASCE 16-95**

Page Revision

- 135 In equation 4.3-3, “ ϕ_s ” should be “ ϕ_s ”.
- 136 In Table 4.3-1, design diameter (D) should be defined as $D = D_1 + X(D_2 - D_1)$.
- 136 In Table 4.3-2, design depth (d) should be defined as $d = d_1 + X(d_2 - d_1)$.
- 137 In 4.5.2, beginning of third paragraph should be, “When the length in bearing, l_b , is no more than 6 in. (150 mm) along the member length and the full bearing length is at least 3 in. (75 mm) from the member end, $F_{c\perp}$ shall be permitted to be multiplied by C_b .”
- 137 In denominator of equation 4.5-7, “ $F_{c\perp}$ ” should be “ $F_{c\perp}$ ”.
- 140 Table 5.2-1 effective length factors should be:

Span Condition	Loading Condition	Bracing Condition	l_e		
			$l_u/d < 7$	$7 \leq l_u/d \leq 14.3$	$l_u/d > 14.3$
Any condition not listed below			$2.06 l_u$	$1.63 l_u + 3d$	$1.84 l_u$
Single span	concentrated load at midspan	braced at ends only	$1.80 l_u$	$1.37 l_u + 3d$	$1.37 l_u + 3d$
	uniformly distributed load	braced at ends only	$2.06 l_u$	$1.63 l_u + 3d$	$1.63 l_u + 3d$
Cantilever	concentrated load at unsupported end	–	$1.87 l_u$	$1.44 l_u + 3d$	$1.44 l_u + 3d$
	uniformly distributed load	–	$1.33 l_u$	$0.90 l_u + 3d$	$0.90 l_u + 3d$
l_e					
Single Span of length L	Uniformly spaced concentrated loads	braced at each concentrated load:			
	one load	$l_u = L/2$	$1.11 l_u$		
	two loads	$l_u = L/3$	$1.68 l_u$		
	three loads	$l_u = L/4$	$1.54 l_u$		
	four loads	$l_u = L/5$	$1.68 l_u$		
	five loads	$l_u = L/6$	$1.73 l_u$		
	six loads	$l_u = L/7$	$1.78 l_u$		
	seven or more loads	–	$1.84 l_u$		
	Equal end moments	–	$1.84 l_u$		

Page **Revision**

143 Section 5.4.2 paragraph 3 should be, “Alternatively, for continuous or cantilevered bending members of sawn lumber, the adjusted shear resistance at locations at least three times the member depth from the member end shall be permitted to be determined using Eq. 5.4-1 or the following:

$$V' = (V' \text{ from Eq. 5.4-1 or 5.4-2}) \left(1 + \frac{(x - 3d)}{3d} \right) \text{ but } \leq 2 (V' \text{ from Eq. 5.4-1 or 5.4-2}) \quad (5.4-3)$$

where x is the distance from the end of the member and C_{rt} is 1.0 for all members when using Eq. 5.4-3.”

152 In section 7.4.3.1, paragraph two, “7.3-1” should be “7.4-1”

156 Equation 7.5-3 should be $Z = (0.93 k_1 D t_s F_{es})/K_{\theta}$.

156 In Table 7.5-2(a), yield mode “III_m” should be “III_m”.

157 In equation 7.5-10, “ F_{tb} ” term should be “ F_{yb} ”.

157 In equation k_4 in Table 7.5-2(c), “ t_s ” term should be “ t_s^2 ”.

167 Equation for K_{gr} should be: $K_{gr} = X - Y(d_c/R_m)$.