



# AMERICAN FOREST & PAPER ASSOCIATION

American Wood Council  
*Engineered and Traditional Wood Products*

November 2009

## 2009 ERRATA to the

## 2005 Edition of

### the *ASD/LRFD STRUCTURAL WOOD DESIGN SOLVED EXAMPLE PROBLEMS* included in the *2005 Wood Design Package*

(printed versions dated 08-06 5M, 02-07 10M, 02-08 10M)

<u>Page(s)</u>	<u>Revision</u>
64	$\text{Defl}_{\text{Max}} = \text{span} \cdot 12 \text{ in/ft} / \cancel{240} \underline{360}$ <span style="color: red;">change 240 to 360 in the denominator</span> $\text{Defl}_{\text{Max}} = \cancel{0.9} \underline{0.6} \text{ in}$
65	$\text{Defl}_{\text{Max}} = \text{span} \cdot 12 \text{ in/ft} / \cancel{240} \underline{360}$ <span style="color: red;">change 240 to 360 in the denominator</span> $\text{Defl}_{\text{Max}} = \cancel{0.9} \underline{0.6} \text{ in}$
66	$I_{\text{reqd}} = \cancel{4037} \underline{1556} \text{ in}^4$ Minimum required I is $\cancel{4037} \underline{1556} \text{ in}^4$ for deflection
67	$I_{\text{reqd}} = \cancel{4037} \underline{1556} \text{ in}^4$ Minimum required I is $\cancel{4037} \underline{1556} \text{ in}^4$ for deflection



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**2009 ERRATA/ADDENDUM  
to the**

**2005 Edition of**

**the *National Design Specification*<sup>®</sup> (*NDS*<sup>®</sup>) for Wood Construction Supplement:  
*Design Values for Wood Construction***

(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

**Page      Revision**

60      Revise footnote 4 of Table 5A, footnote 3 of Expanded Table 5A, and footnote 3 of Table 5B as follows:

The design value for shear,  $F_{vx}$  and  $F_{vy}$ , shall be decreased by multiplying by a factor of 0.72 for non-prismatic members, notched members, and for all members subject to impact or cyclic loading. The reduced design value shall be used for design of members at connections (NDS 3.4.3.3 and 10.1.2) that transfer shear by mechanical fasteners. The reduced design value shall also be used for determination of design values for radial tension (NDS 5.2.2).



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**2009 ERRATA/ADDENDUM  
to the**

**2005 Edition of**

**the Commentary to the *National Design Specification*<sup>®</sup> (*NDS*<sup>®</sup>) for Wood Construction**  
(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

**Page    Revision**

249    Revise equation C15.1-1 as follows:

$$P_{14} = -180.7 + 140.5 \log(P_m)$$



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<u>Page(s)</u>	<u>Revision</u>	
264	$M_{\text{Total}} = M_{\text{load}} + \text{Weight} \cdot \text{Length}^2 / 8 \cdot \underline{12}$	delete "12" in the denominator
	$f_b = \underline{1695} \underline{1778}$ psi	
265	$M_{\text{Total}} = M_{\text{load}} + 1.2(\text{Weight} \cdot \text{Length}^2) / 8 \cdot \underline{12}$	delete "12" in the denominator
	$f_b = \underline{2575} \underline{2674}$ psi	
274	$M_{\text{Total}} = M_{\text{load}} + \text{Weight} \cdot \text{Length}^2 / 8 \cdot \underline{12}$	delete "12" in the denominator
	$f_b = \underline{1352} \underline{1401}$ psi	
275	$M_{\text{Total}} = M_{\text{load}} + \underline{1.2}(\text{Weight} \cdot \text{Length}^2) / 8 \cdot \underline{12}$	delete "12" in the denominator and incorporate 1.2 load factor
	$f_b = \underline{2145} \underline{2204}$ psi	



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*the National Design Specification® (NDS®) for Wood Construction*  
(printed versions dated 04-05 2M, 09-05 2M, 08-06 5M, and 02-08 10M)

The 1991-2005 Editions of the *National Design Specification®(NDS®) for Wood Construction* have contained provisions for designing columns loaded simultaneously with axial, edgewise bending, and/or flatwise bending loads using a stress interaction equation. When a flatwise bending load is checked with the third term of the stress interaction equation, the axial and edgewise bending interaction in the denominator can become a negative value. The occurrence of the negative value indicates an overstress. Use of this negative term in the stress interaction equation overlooks the overstress in flatwise bending and incorrectly reduces the overall interaction.

While a check for overstress due to bending is a limiting condition of member design for bending per 3.3.1 of the *NDS*, an explicit limit is provided to clarify limitations on flatwise bending in *NDS* stress interaction equations as follows:

**Page**    **Revision**

20-21    Add limitation to provisions in *NDS* 3.9.2:

$$\frac{f_c}{F_{cE2}} + \left( \frac{f_{b1}}{F_{bE}} \right)^2 < 1.0$$

**Page**    **Revision**

190    Append to *NDS Commentary* C3.9.2:

The limits on  $f_c$  and  $f_{b1}$  (e.g.  $f_c < F_{cE1}$ ,  $f_c < F_{cE2}$ , and  $f_{b1} < F_{bE}$ ) do not address the case where the sum of the  $f_c/F_{cE2}$  stress ratio and the square of the  $f_{b1}/F_{bE}$  stress ratio in the denominator of the third term of Equation 3.9-3 exceeds 1.0. In this case, the third term becomes negative indicating overstress in flatwise bending due to combined loading effects. Inclusion of the third term as a negative value overlooks the overstress in flatwise bending and incorrectly reduces the interaction calculation. A limit on  $(f_c/F_{cE2}) + (f_{b1}/F_{bE})^2$  is stated explicitly to clarify limitations on flatwise bending in the stress interaction equation and avoid accidental inclusion of a negative value in the stress interaction equation.

**Page**    **Revision**

137    Add limitation to provisions in *NDS* 15.4.1:

$$\frac{f_c}{F_{cE2}} + \left( \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right)^2 < 1.0$$

**Page**    **Revision**

253    Append to *NDS Commentary C15.4.1*:

The limits on  $f_c$  and  $f_{b1}$  (e.g.  $f_c < F_{ce1}$ ,  $f_c < F_{ce2}$ , and  $f_{b1} < F_{be}$ ) do not address the case where the sum of the  $f_c/F_{ce2}$  stress ratio and the square of the  $f_{b1}/F_{be}$  stress ratio or the  $[f_{b1} + f_c(6e_1/d_1)]/F_{be}$  ratio in the denominator of the third term of Equations 15.4-1 and 15.4-2 exceeds 1.0. In this case, the third term becomes negative indicating overstress in flatwise bending due to combined loading effects. Inclusion of the third term as a negative value overlooks the overstress in flatwise bending and incorrectly reduces the interaction calculation. A limit on  $(f_c/F_{ce2}) + ([f_{b1} + 6f_c e_1/d_1]/F_{be})^2$  is stated explicitly to clarify limitations on flatwise bending in the stress interaction equation and avoid accidental inclusion of a negative value in the stress interaction equations.



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### the *SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC, ANSI/AF&PA SDPWS-2005* included in the *2005 Wood Design Package*

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<u>Page(s)</u>	<u>Revision</u>	
47	In Example C4.2.2-1 revise “Nail load/slip at $1.4 v_{s(ASD)}$ .”	Subscript (ASD)
	In Example C4.2.2-1 revise “= $(129.5/456)^{3.144} = 0.0191$ ”	Superscript 3.144
50	In Equations C4.3.2-1 and C4.3.2-2 revise “(tie-down <del>nail</del> slip)”	Delete “nail”
	In the third paragraph of C4.3.2 (second column) revise “...or 3-term equation (SDPW <del>D</del> S)...”	Change “D” to “S”
53	In Example C4.3.2-1, text beginning with “C4.3.2.1” to the end should be moved to page 51 at the end of section C4.3.2.	



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**Page(s)**

**Revision**

155

Revise Table M16.2-10 column headers for decking width as follows:

Rating	1-HOUR				1.5-HOUR			2-HOUR	
Decking Width	<del>5</del> <sub>1.5</sub>	<del>7</del> <sub>2.5</sub>	<del>9</del> <sub>3.5</sub>	<del>11</del> <sub>5.5</sub>	<del>7</del> <sub>2.5</sub>	<del>9</del> <sub>3.5</sub>	<del>11</del> <sub>5.5</sub>	<del>9</del> <sub>3.5</sub>	<del>11</del> <sub>5.5</sub>



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**Page(s)**

64

**Revision**

Figure M9.2-4 should be renumbered M9.2-5

Figure M9.2-5 should be renumbered M9.2-4



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<u>Page(s)</u>	<u>Revision</u>	
72	$C_{M\_Fb} := \underline{0.85} \underline{1.0}$	Change wet service factor. According to the <i>NDS Supplement</i> , when $(F_b)(C_F) \leq 1150$ psi $C_M = 1.0$
	$F'_{bx} = \underline{4124} \underline{1322}$ psi	This change does not affect the outcome - bending still checks.
73	$C_{M\_Fb} := \underline{0.85} \underline{1.0}$	Change wet service factor. According to the <i>NDS Supplement</i> , when $(F_b)(C_F) \leq 1150$ psi $C_M = 1.0$
	$F'_{bx} = \underline{2428} \underline{2856}$ psi	This change does not affect the outcome - bending still checks.



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<u>Page(s)</u>	<u>Revision</u>
91	$\lambda = \del{1.0} \underline{0.8}$ Time effect factor
93	$F'_{bs\_24\_0} = \del{648} \underline{518}$ lbf·in $F'_{bs\_24\_16} = \del{832} \underline{665}$ lbf·in $F'_{bs\_32\_16} = \del{964} \underline{769}$ lbf·in $F'_{bs\_40\_20} = \del{1620} \underline{1296}$ lbf·in $F'_{bs\_48\_24} = \del{2160} \underline{1728}$ lbf·in  <b>The maximum moment is 826 in-lb/ft, so the <del>24/16</del> <u>40/20</u> span rating works for flexure.</b>
95	Check the <del>24/16</del> <u>40/20</u> span rating for shear.  $F'_{s\_lbQ} := \del{150} \underline{205}$ lbf  $F'_{s\_lbQ} = \del{324} \underline{354}$ lbf  <b>The <del>24/16</del> <u>40/20</u> span rating is OK in shear.</b>  $EI_{40\_20} := \del{78000} \underline{225000}$ lbf·in <sup>2</sup> $EI = \underline{225000} \del{78000}$ lbf·in <sup>2</sup> per foot of width for <del>24/16</del> <u>40/20</u> span rating. Table M9.2-1.  $EI'_{40\_20} := \del{78000} \underline{225000}$ lbf·in <sup>2</sup> <del>24/16</del> <u>40/20</u> bending stiffness
96	$\Delta < L/240 = \del{0.95} \underline{0.095}$ in, thus deflection is OK
97	$\Delta = 0.00677 w_{load} span_{defl}^4 / EI'_{40\_20}$  $\Delta = \del{0.213} \underline{0.074}$ in



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<u>Page(s)</u>	<u>Revision</u>
376	$\text{Weight}_{\text{curved}} = \pi \cdot R_{\text{center}}^{\cancel{2}} \cdot \text{Depth} \cdot \text{Width} \cdot 40 \cdot \text{lbf/ft}^3$ (delete "2" in the denominator)  $\text{Weight}_{\text{curved}} = 353 \underline{706} \text{ lbf}$  $M_{\text{max}} = 66244 \underline{97288} \text{ in} \cdot \text{lbf}$
377	$\text{Weight}_{\text{curved}} = \pi \cdot R_{\text{center}}^{\cancel{2}} \cdot \text{Depth} \cdot \text{Width} \cdot 40 \cdot \text{lbf/ft}^3$ (delete "2" in the denominator)  $\text{Weight}_{\text{curved}} = 353 \underline{706} \text{ lbf}$  $M_{\text{max}} = 92742 \underline{136204} \text{ in} \cdot \text{lbf}$
380	$f_b = 343 \underline{504} \text{ psi}$  $V_{\text{load}} = 376 \underline{553} \text{ lbf}$  $f_v = 5 \underline{8} \text{ psi}$  $f_r = 40 \underline{15} \text{ psi}$
381	$f_b = 481 \underline{706} \text{ psi}$  $V_{\text{load}} = 527 \underline{774} \text{ lbf}$  $f_v = 7 \underline{11} \text{ psi}$  $f_r = 14 \underline{21} \text{ psi}$

384  $\text{Weight}_{\text{curved}} = \pi \cdot R_{\text{center}}/2 \text{ Depth} \cdot \text{Width} \cdot 40 \cdot \text{lbf/ft}^3$  (delete "2" in the denominator)

$\text{Weight}_{\text{curved}} = \cancel{353} \underline{706} \text{ lbf}$

$M_{\text{max}} = \cancel{66244} \underline{97288} \text{ in} \cdot \text{lbf}$

$P_{\text{max}} = \cancel{553} \underline{906} \text{ lbf}$

385  $\text{Weight}_{\text{curved}} = \pi \cdot R_{\text{center}}/2 \text{ Depth} \cdot \text{Width} \cdot 40 \cdot \text{lbf/ft}^3$  (delete "2" in the denominator)

$\text{Weight}_{\text{curved}} = \cancel{353} \underline{706} \text{ lbf}$

$M_{\text{max}} = \cancel{92742} \underline{136204} \text{ in} \cdot \text{lbf}$

$P_{\text{max}} = \cancel{774} \underline{1268} \text{ lbf}$

388  $f_c = \cancel{5} \underline{8} \text{ psi}$

389  $f_c = \cancel{7} \underline{11} \text{ psi}$

390  $f_{b1} = \cancel{343} \underline{504} \text{ psi}$

$$\left(\frac{f_c}{F_c}\right)^2 + \frac{f_{b1}}{F_b \left[1 - \left(\frac{f_c}{F_{cE1}}\right)\right]} = \cancel{0.314} \underline{0.469}$$

391  $f_{b1} = \cancel{481} \underline{706} \text{ psi}$

$$\left(\frac{f_c}{F_c}\right)^2 + \frac{f_{b1}}{F_b \left[1 - \left(\frac{f_c}{F_{cE1}}\right)\right]} = \cancel{0.305} \underline{0.454}$$

392  $\left(\frac{f_c}{F_c}\right)^2 = \cancel{0.00056} \underline{0.00151}$   $\frac{f_{b1}}{F_b \left[1 - \left(\frac{f_c}{F_{cE1}}\right)\right]} = \cancel{0.313} \underline{0.467}$

393  $\left(\frac{f_c}{F_c}\right)^2 = \cancel{0.00049} \underline{0.00132}$   $\frac{f_{b1}}{F_b \left[1 - \left(\frac{f_c}{F_{cE1}}\right)\right]} = \cancel{0.304} \underline{0.453}$



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#### Page(s)

#### Revision

276

$$F'_v = \cancel{2/3} F_{vx} C_D C_M C_{t,Fv}$$

$$F'_v = \cancel{200} \underline{300} \text{ psi}$$

This change does not affect the final results for shear calculations since  $f_v < F'_v$ .

277

$$F'_v = \cancel{2/3} \lambda K_{F,Fv} \phi_v F_{vx} C_M C_{t,Fv}$$

$$F'_v = \cancel{345.6} \underline{518.4} \text{ psi}$$

This change does not affect the final results for shear calculations since  $f_v < F'_v$ .



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<b><u>Page(s)</u></b>	<b><u>Revision</u></b>
126	Change the IIC rating for assemblies “Without Gypsum Concrete” with “Carpet & Pad” from <del>62</del> to <u>66</u> .
129	Change the STC rating for assemblies “With Gypsum Concrete” with “Carpet & Pad” from <del>68</del> <sup>c</sup> to <u>58</u> <sup>c</sup> .



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***the National Design Specification® (NDS®) for Wood Construction Supplement: Design Values for Wood Construction***

(printed versions dated 04-05 2M, 09-05 2M, and 08-06 5M)

**Page Revision**

4 In Table 2.1, revise the following:

Species or Species Combination	Species That May Be Included in Combination	Grading Rules Agencies	Design Values Provided in Tables
Coast Sitka Spruce	Coast Sitka Spruce	NLGA	<u>4A</u> , 4D, 4E
<u>Yellow Cedar</u>	<u>Yellow Cedar</u>	<u>NLGA</u>	<u>4A</u>

32 In Table 4A, add the following design values for Coast Sitka Spruce

Species and commercial grade	Size classification	Design values in pounds per square inch (psi)							Grading Rules Agency
		Bending F <sub>b</sub>	Tension parallel to grain F <sub>t</sub>	Shear parallel to grain F <sub>v</sub>	Compression perpendicular to grain F <sub>c⊥</sub>	Compression parallel to grain F <sub>c</sub>	Modulus of Elasticity		
							E	E <sub>min</sub>	
<b>Coast Sitka Spruce<sup>4</sup></b>									
Select Structural No. 1 / No. 2	2" & wider	1300	950	125	455	1200	1,700,000	620,000	NLGA
No. 3		925	550	125	455	1100	1,500,000	550,000	
Stud	2" & wider	525	325	125	455	625	1,400,000	510,000	
Construction Standard		725	450	125	455	675	1,400,000	510,000	
Utility		1050	650	125	455	1300	1,400,000	510,000	
	2"-4" wide	600	350	125	455	1100	1,300,000	470,000	
		275	175	125	455	725	1,200,000	440,000	

34 In Table 4A, revise the following design values for Northern Species

Species and commercial grade	Size classification	Design values in pounds per square inch (psi)							Grading Rules Agency
		Bending F <sub>b</sub>	Tension parallel to grain F <sub>t</sub>	Shear parallel to grain F <sub>v</sub>	Compression perpendicular to grain F <sub>c⊥</sub>	Compression parallel to grain F <sub>c</sub>	Modulus of Elasticity		
							E	E <sub>min</sub>	
<b>Northern Species</b>									
Select Structural No. 1 / No. 2	2" & wider	<del>4,000</del> <u>975</u>	<del>450</del> <u>425</u>	110	350	1,100	1,100,000	400,000	NLGA
No. 3		<del>600</del> <u>625</u>	275	110	350	850	1,100,000	400,000	
Stud	2" & wider	350	150	110	350	500	1,000,000	370,000	
Construction Standard		475	225	110	350	550	1,000,000	370,000	
Utility		700	<del>300</del> <u>325</u>	110	350	1,050	1,000,000	370,000	
	2"-4" wide	400	175	110	350	875	900,000	330,000	
		175	75	110	350	575	900,000	330,000	

**Page Revision**

36 In Table 4A, add the following design values for Yellow Cedar

Species and commercial grade	Size classification	Design values in pounds per square inch (psi)							Grading Rules Agency
		Bending F <sub>b</sub>	Tension parallel to grain F <sub>t</sub>	Shear parallel to grain F <sub>v</sub>	Compression perpendicular to grain F <sub>cL</sub>	Compression parallel to grain F <sub>c</sub>	Modulus of Elasticity		
							E	E <sub>min</sub>	
<b>Yellow Cedar<sup>4</sup></b>									
Select Structural		1200	725	175	540	1200	1,600,000	580,000	NLGA
No. 1 / No. 2	2" & wider	800	475	175	540	1000	1,400,000	510,000	
No. 3		475	275	175	540	575	1,200,000	440,000	
Stud	2" & wider	625	375	175	540	650	1,200,000	440,000	
Construction		925	550	175	540	1200	1,300,000	470,000	
Standard	2"-4" wide	525	300	175	540	1050	1,200,000	440,000	
Utility		250	150	175	540	675	1,100,000	400,000	

36 In Table 4A, add the following footnote:

**4. SPECIFIC GRAVITY, G.** Specific gravity values are provided below for visually graded dimension lumber. Note that the value for Coast Sitka Spruce is applicable only for visually graded dimension lumber (2" – 4" thick). See NDS Table 11.3.2A for the specific gravity value applicable to Coast Sitka Spruce used as visually graded timber (5"x5" and larger) and visually graded decking.

Species	Specific Gravity, G	Grading Rules Agency
Coast Sitka Spruce	0.43	NLGA
Yellow Cedar	0.46	NLGA



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<u>Page(s)</u>	<u>Revision</u>
6	x = distance from chord splice to nearest support, <del>in</del> <u>ft</u>
13	x = distance from chord splice to nearest support, <del>in</del> <u>ft</u>
23	$C_o$ = shear capacity adjustment factor from Table 4.3.2.1 <u>4.3.3.4</u>
44	x = distance from chord splice to nearest support, <del>in</del> <u>ft</u>
46	x = distance from chord splice to nearest support, <del>in</del> <u>ft</u>



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**Page(s)**  
 120-121

**Revision**

The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} \leq 1.0$$

The square symbol in the denominator goes between the last 2 brackets in the equation.

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The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\left( \frac{f_c}{F_c} \right)^2 + \frac{f_{b1} + f_c(6e_1/d_1) [1 + 0.234(f_c/F_{cE1})]}{F_{b1} [1 - (f_c/F_{cE1})]} + \frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} = \underline{0.945} \quad \underline{0.927}$$

The square symbol in the denominator goes between the last 2 brackets in the equation. This changes the overall result to 0.927 (above) and 0.101 for the minor axis bending term (below).

$$\frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} = \underline{0.119} \quad \underline{0.101}$$

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**Revision**

The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\left(\frac{f_c}{F_c}\right)^2 + \frac{f_{b1} + f_c(6e_1/d_1)[1 + 0.234(f_c/F_{cE1})]}{F_{b1}[1 - (f_c/F_{cE1})]} + \frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} = \underline{0.785} \quad \underline{0.772}$$

The square symbol in the denominator goes between the last 2 brackets in the equation. This changes the overall result to 0.772 (above) and 0.142 for the minor axis bending term (below).

$$\frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} = \underline{0.156} \quad \underline{0.142}$$

284-285 The third part (minor axis bending component) of the biaxial bending and compression interaction equation should be revised as follows:

$$\frac{f_{b2} + f_c(6e_2/d_2) \left\{ 1 + 0.234(f_c/F_{cE2}) + 0.234 \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}}{F_{b2} \left\{ 1 - (f_c/F_{cE2}) - \left[ \frac{f_{b1} + f_c(6e_1/d_1)}{F_{bE}} \right]^2 \right\}} \leq 1.0$$

The square symbol in the denominator goes between the last 2 brackets in the equation.